

Digital Surface Model (DSM) Construction and Flood Hazard Simulation for Development Plans in Naga City, Philippines

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Abstract

A 2D-hydraulic flood propagation models require accurate elevation data. One of the main problems is frequent changes of land use in major cities, where frequent updating of the digital terrain model (DTM) for flood modelling might be needed. On the other hand the assessment should be based on realistic flood hazard indicator that would help to reflect the real impact of urban development on the surrounding areas. This paper presents an example of assessing the impact of flood for future developments in Naga City, the Philippines. The elevation data is constructed through integrating various elevation data derived from many sources. The development impact assessment begins with the detailed observation on changes in flood characteristics. This is supported by the analyses on the community-based flood risk perception and investigation on changes of flood hazard (based on the flood velocity and depth).

In the DTM construction the natural terrain is separated from the man-made terrain. The geostatistical approach is used to investigate the effect of integrating multi-sources of elevation data by evaluating the nugget values. The data sources are prioritized based on the nominal horizontal and vertical accuracy, and form of data. In this paper, there are 4 interpolation methods used, namely Australian National University's Digital Elevation Model algorithm (ANUDEM), Kriging, Polynomial and Triangulated Irregular Network (TIN). The assessments are based on percentile vertical accuracy assessment, error point's distribution and visual assessment. As a result, the kriging interpolation method has produced the best DTM and it full-filled the requirements for hydrological flood modelling purpose. Finally the Digital Surface Model (DSM) of the study area was constructed by integrating both man-made and natural terrains. The DSM was also generated to simulate the new developments in Naga City. The 1D2D SOBEK flood model was used to simulate flood events for 2, 5, 10 and 17.5 years return period flood. In addition, the flood depths and flood extent during Supertyphoon Nanmadol were used in flood model calibration.

Flood calibration results revealed that the calibrated flood model was able to simulate the real flood event up to 0.35 m accuracy of flood depth. In the development impact assessment, it was found that the impact of the developments is larger for a larger flood magnitude. Furthermore the pattern of the changes in flood behaviour depends on the location from the main developments. The Almeda Highway acted as a barrier, that obstructs the flood water from go farther. In addition the small scale construction, for instance the Drainage System in Barangay Triangulo had played a major role in changing the flood behaviour, especially in a small magnitude flood. Through this study, it was proved that by simply elevating ground terrain only can solve the flood problem in a particular area. However, the flood problem is transferred to another area.

1. INTRODUCTION

Rapid and uncontrolled urbanization in developing countries has become one of the major issues in hazard and risk management. This is certainly one of the major environmental problems in the developing world, today and in the years to come. A huge concentration of people, business activities and properties has made hazard management in urban area more difficult and complex. With an increased value of property, for instance, buildings and other structures, potential damage from

prolonged and severe flooding can easily extend into the million of dollars. Besides, flooding in crowded area due to rapid urbanization would dramatically increase the loss. Urbanization has a great influence on rainfall runoff and flood behaviour. The flow of the floodwater becomes complex as a result of complicated buildings distribution and structures in an urban area. Heavy rainfall is easily converted to run-off over paved surface, and due to improper urbanization planning, water will accumulate and increase the potential of flooding. As a consequence, it is a great challenge to forecast urban flooding and calculate the potential damage. Improper development planning, in developing countries, might ignore its impact on flood hazard and risk to the surrounding community.

Advancement in computer processing power, accurate terrain data acquisition and the integration between 1 dimensional (1D) and 2 dimensional (2D) flood modelling make possible to model dynamic flooding in a complex urban environment. The 2D flood modeling requires information on terrain, which quality depends on the acquisition techniques and the terrain of the study area. Complex and densely populated urban areas require more detailed terrain elevation data compared to rural area. In urban areas, the characteristics of floodwater flow are controlled by the distribution of buildings, roads, elevated area and etc. This requires the basis on defining the spatial resolution and accuracy of Digital Terrain Model (DTM). Changes in urban area as a result of urbanization can be simulated through modification of the existing DTM and/or land use or land cover information.

The main objective of this research is to generate DTM and DSM of the study area with consideration on current and future developments, followed by simulating the flood events and development impact assessment. The development impact assessment was made based on three methods; 1) Changes in flood characteristics 2) Changes in flood risk perception by the Naga City Community and 3) Changes in flood hazard area.

2. STUDY SITE

Naga City is located in Bicol region, at the south-eastern tip of the Philippine island of Luzon. Naga City, located about 377 km to the south of Manila, is well known as a fast-growing area (see figure 2.1). Naga City has the largest population among 35 municipalities in Camarines Sur, which population covers about 8.9 percent of the total population of the province (Naga City Government Philippines Business for Social Progress, 2001). Naga is considered the heart of the Bicol region, consists of 27 Barangays on the land of 7,748 hectares. The main portion of The Naga city is located in low and flat topography that usually inundated by flood when water from the Naga and Bicol River overflow. Thus, substantial discharge and heavy rainfall during monsoon commonly causes severe flood in this city.

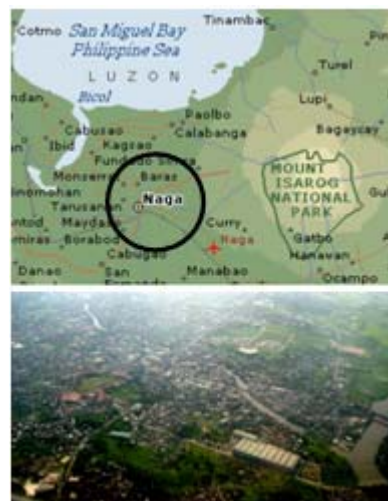


Figure 2.1: Naga City

Apart from the current developments in Naga City, several other developments are being introduced and aimed at giving better facilities for the community. According to the Naga City Development Plan, the developments are divided into 6 main zones; 1) Central Business District I (CBD I), 2) Central Business District II (CBD II), 3) South Riverfront Growth Area, 4) Concepcion Growth Corridor, 5) East Highland Tourism Zone and 6) Naga City Agro-Industrial Zone. Some of these zones are located in flood prone

area, for instance CBD II.

3. DATA COLLECTION RESEARCH METHODOLOGY

In this research the data collection is divided into 4 major groups as follow.

1. Elevation or topographical data
2. Landuse or landcover
3. Recent and future developments
4. Rainfall and floodwater depth during the Super Typhoon Nanmadol.

Apart from the available topographical data, additional elevation information is needed to fill gaps in the available data and to update terrain information due to recent and future developments. Other necessary data for instance Landuse or Landcover, Rainfall, flood depth and extent and flood risk perception are needed in flood modeling and development impact assessment.

This study is divided into 4 main phases, namely, 1) Data preparation and analysis, 2) DTM and DSM modelling, 3) flood model calibration and modelling and 4) development impact assessment (see figure 3.1). The first phase focuses on data preparation, analysis and integration. The main data is divided into landuse and landcover, flood information, terrain data, hydrological data and flood hazard data. The second phase of the research methodology concerns on the construction of DTM and DSM based on different sources of elevation data, which were derived from both primary and secondary sources. The primary data collection aims at filling the gaps in the available data and to update terrain changes in the study area as a result of recent developments. In general, the elevation data is derived in various forms, for instance points, line and polygons, and these data are then aggregated into ground terrain and man-made features. Further aggregation is made to separate the elevation data into two terrain situations; current and future situations. The DTMs are produced using different terrain interpolation methods. The best product is selected and integrated with man –made features to produce DSMs of the study area.

The 1D2D SOBEK flood model is used to simulate 5 recurrence intervals flood events. The flood calibration is made base on flood depth information derived from recent field observations (this data were collected by Saut Sagala and Peters Guarin Graciela) after the flood event caused by the Super typhoon Nanmadol (with an equivalent to 10 years return period flood). The flood depth information was collected through interviews with the local community in Barangay Triangulo and Barangay Sabang few months after the flood event. The flood calibration is based on two aspects; surface roughness and building structure. The calibrated surface roughness and the suitable building representation will be used for further flood modelling. The surface roughness value of the study is based on landuse or landcover. This updated information is used together with development plans to create recent and future landuse or landcover in Naga City.

The final phase emphasizes on development impact assessments based on detailed investigation of flood characteristics before and after the development. This is supported by additional assessment focuses on changes of flood hazard areas for current and future situation of Naga City. The definition of flood hazard is based on the combination of flood velocity and flood depth.

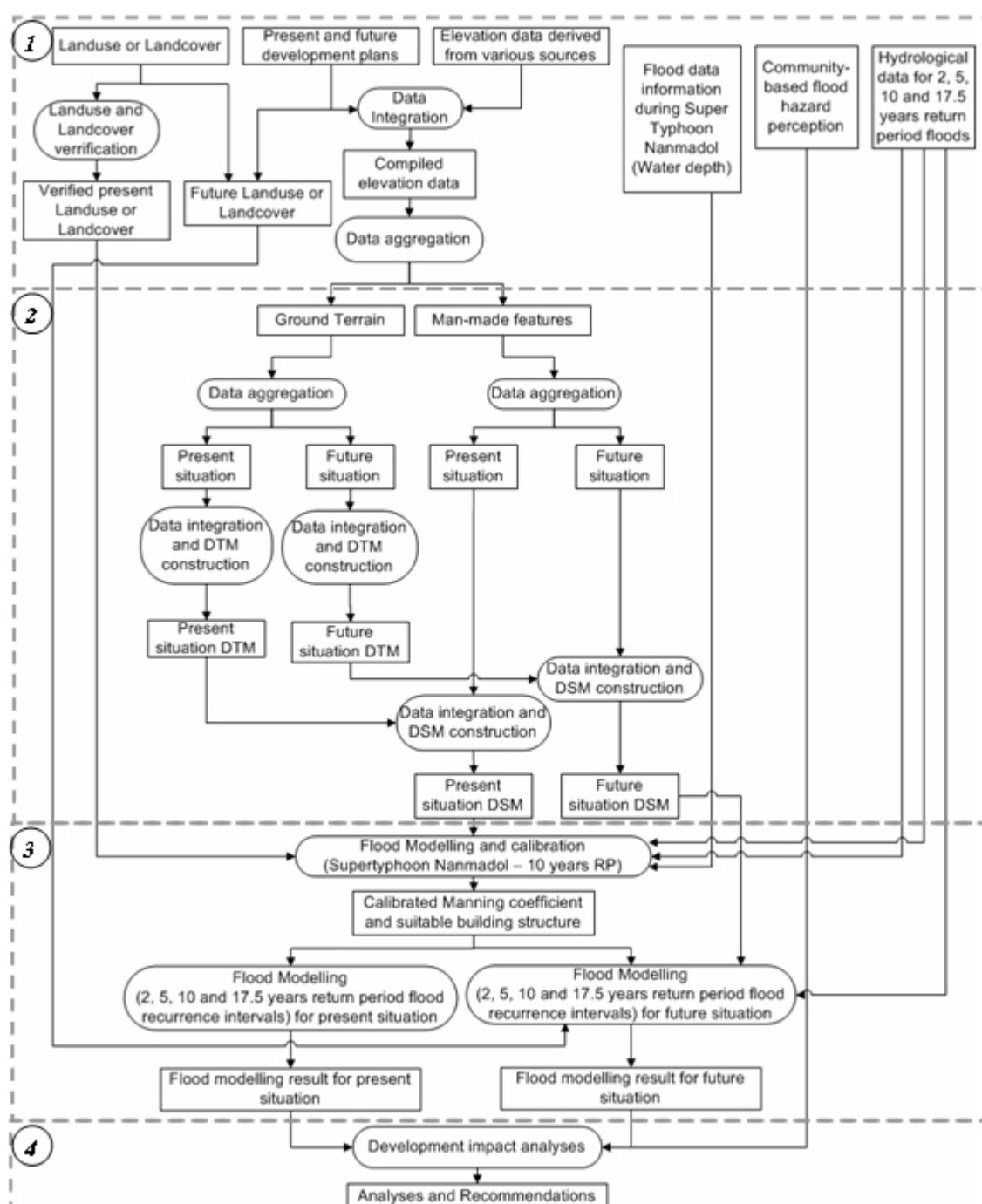


Figure 3.1: Overall research methodology

3.1 DTM and DSM construction

DTM and DSM of the study area were generated in 4 major steps; 1) elevation data preparation and analysis, 2) elevation data interpolation, 3) accuracy assessment and reporting and finally 4) integrating natural terrain with man made terrain to produce DSM.

3.1.1 Elevation data preparation and analysis

The elevation dataset for DTM generation is derived through the integration of various elevation data sources and these data vary in both horizontal and vertical accuracies (see appendix 1). Blomgren (1999), in his study used a rectilinear grid over the contour lines to transfer elevation information from contour lines to point forms. The point was digitized as close as possible to the overlaid grid. Therefore,

more evenly distributed elevation samples were derived, and it improved the interpolation performance (Blomgren, 1999). On the other hand, the arrangement of elevation data will influence the shape of the variogram model. In this case, the clustered elevation data, which were found around dunes, road embankments and other local abrupt changes in topography, were removed from the dataset (Blomgren, 1999). Wilson and Atkinson (2003) in their research, "Prediction the uncertainty of DEM on flood inundation modelling", used the ordinary kriging to interpolate the elevation data in the floodplain area. The elevation data was the combination between the contour lines and the elevation points that were derived from GPS measurement. The original experimental semi-variogram of the contour lines had quite general shape. However this general shape or trend was reduced (increased variance at shorter lags than globally) when the elevation points derived from GPS measurements were added to the dataset.

Ten set elevation data were used and integrated to produce DTM. These datasets vary in scale and contour interval which remarks difference in horizontal (planimetric) and vertical accuracies. Besides the elevation information derived from the available topographical maps, additional elevation measurements using geodetic levelling were done to fill the gaps and update terrain information of recently developed area. The problem of integrating elevation data from different sources with different scales and accuracies lies on the fact that the elevation values in the combined dataset may lie close to each other. The challenge is to identify an appropriate approach to prioritize the datasets, to identify which of those datasets represent the true terrain elevation and to combine the entire datasets. Thus, the datasets are prioritized with 2 steps; 1) Prioritization based on Nominal horizontal and vertical accuracies (based on the National Standard Data Accuracy (NSSDA)), 2) Prioritization based on data forms (spot heights and contour lines) and production date (see table 3.1).

	Source of data	Priority level based on the nominal accuracy	Priority level based on the 2 nd selection step	Final priority level
1	Contour line south of Naga City	4	3	6
2	Contour line for the whole Naga city	3	3	5
3	Contour line from Naga Drainage Plan (1981)	2	3	4
4	Contour line from CBD II development Plan	2	3	4
5	Spot height from the topographical map south of Naga City	3	2	3
6	Spot height along roads from Naga City Drainage Plan (1981)	1	2	2
7	Spot height from Drainage Plan in Triangulo (ground and drainage crown elevation)	1	2	2
8	Almeda highway plan (ground and final road elevation)	1	2	2
9	Spot height for the whole Bicol region	6	2	7
10	Field observation spot height	5	1	1

Table 3.1: Available elevation data sources for DTM generation

The contour lines are converted to points and then combined with other point form data (spot heights). Data with higher priority score would replace the lower priority score data. The replacement is done when 2 or more elevation points fall within 3 meters radius. The effect of the integration of the multi-sources elevation data is assessed by mean of semi-variogram analysis. The assumption is points that are close together should have less difference or high autocorrelation. Thus, high nugget value would

reveal strong effect of disagreement between the elevation datasets. Certainly, the nugget effect could also attribute to the complexity of terrain features. However it was found that, the effect of data disagreement still appear when datasets with complex geomorphological features are removed. Several attempts with different integration method were used (see table 3.2) to decrease the nugget value. However, for the sake of the terrain complexity information, the nugget value was reduced from 2.9 to 2.2.

Dataset	Nugget	Sill	Range	Lag	Model	Number of points
50 m contour lines to point conversion interval (Dataset B)	2.9	52	4500	150 m	Gaussian	11,131
100 m contour lines to point conversion interval (Dataset A)	2.2	42	4500	150 m	Gaussian	6889
5 m block elevation average	2.2	37	4500	150 m	Gaussian	5566

Table 3.2: The value of Semi-variogram model parameters for each dataset; the 2nd and the 3rd datasets will be used for the DTM interpolation

At this stage, the integrated elevation datasets with low nugget value are assumed to have less disagreement between elevation dataset, less overlapping dataset, good elevation data in representing the real terrain and inevitably contain less degraded complex terrain features.

3.1.2 Elevation data interpolation

The DTM interpolation method should in general preserve the detailed terrain information while reducing the effect datasets disagreement. The interpolation of the ground terrain is done with 4 interpolation techniques; 1) Kriging, 2) TIN, 3) Polynomial trend surface and 4) ANUDEM (see figure 3.2).

A Geostatistical interpolation or kriging interpolation method is similar to a probabilistic interpolation technique, in which the weights are derived from the surrounding sample points. However, the weights are not only based on the distance, but also on the strength of the overall correlation among the measured points (Maune, 2001). The basic interpolation assumption is that, values at a short distance are more likely to be similar than at a larger distance. Demirhan et al., (2003) in his study had focused on the performance of several interpolation methods with presence of noise and sampling pattern. He had pointed out that the "Ordinary Kriging is the most robust interpolations method against noise". On the other hand, the deterministic DTM interpolation approach tries to fit a mathematical function to a set of elevation samples of known coordinate (x and y). This interpolation technique could be done either through an exact interpolation or smooth interpolation (Meijerink et al., 1994).

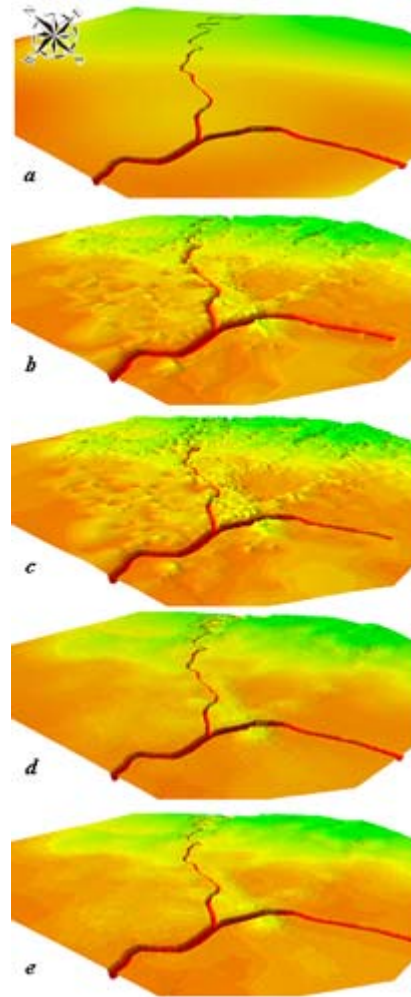


Figure 3.2: Natural terrain derived from 6th Polynomial degree (a), TIN-based terrain modelling (b), ANUDEM (c), Ordinary Kriging (100 m conversion interval) (d), and Ordinary Kriging (5 m average block) (e), visualized in 3D

The smooth interpolation method is suitable with the assumption that point measurement data are regarded as true value or with less error. On the other hand, with coarse data accuracy, the smooth interpolation scheme might be the best way to level out the error to some degree. The fitting process uses various types of mathematical functions usually known as polynomial functions at a certain degree of complexity to fit the surface through the sample points.

The nature of TIN modelling is splitting the surface into triangular element planes (Meijerink et al., 1994). More detailed definition, "TIN is a digital terrain model based on irregular array of points which forms a sheet of non-overlapping contiguous triangle facets" (Maune, 2001). It is a vector model that supports lines, points and area-based features in representing the surface morphology. The next interpolation technique is ANUDEM. ANUDEM is a software package known as the Australian National University Digital Elevation Model developed by Hutchinson (Geodata and Geoscience Australia, 2002). The ANUDEM interpolator was designed and optimised to create a hydrologically correct terrain models. "It's unique in both input and output for building a good terrain model"(Maune, 2001). The input data is not only confined to point data, but also lines which represent streams and ridges for drainage, and polygons as a lake boundary to produce a DEM that is virtually free of spurious sinks and pits

3.1.3 DTM quality assessment and reporting

The DTM quality assessment comprises of 3 main areas; 1) DTM accuracy assessment 2) DTM errors distribution and 3) General geomorphological of the study area. The DTM accuracy assessment uses Percentile Accuracy Assessment method introduced by (Maune, 2001) which is suitable for non-normally

distributed residual data. According to table 3.3, ANUDEM interpolation method has produced the most accurate DTM with the maximum error 0.98 m at 80 percent of the total residual data. Appendix 2 shows complete results of the DTMs' accuracy produced by each interpolation technique. The DTM produced by the Ordinary Kriging with 5 m block average has the second lowest error with maximum error 1.00 m. followed by DTM produced by Ordinary kriging on 100 m contour lines to point conversion and finally the polynomial interpolation methods.

The flood modelling gives more focus in the low-lying areas, where most of the commercial areas are located. Thus, the next assessment focuses on the number of point with errors more than 1.0 within area of interest (see appendix 3). It was found that, TIN-based surface modelling has the least number of points with error more than 1.0 m and followed by the Ordinary Kriging with 5 m block average, the Ordinary Kriging with 100 m contour lines to points' conversion interval, ANUDEM and Polynomial. The final stage in DTM quality assessment focuses on the capability of DTMs in representing the real landscape of the study area. According to figure 3.2, the DTM produced by the Ordinary Kriging with 5 meters elevation block average is able to represent the real landscape of the study area. The DTM has shown in general, backswamp areas, natural levee and elevated areas for bridge construction. However, another DTM produced for instance by ANUDEM and TIN contain large number of artificial pits. According to Raaflaub and Collins (2005) artificial pits or sink features in DTM are hydrologically serious problem. In fact, "Pits generally appear in flatter area where even a one meter error can be enough to produce a close depression"(Raaflaub and Collins, 2005). On the other hand, DTMs produced by the Polynomial interpolation method shows smooth surface and failed to represent the general landscape of the study area. Therefore based on above judgements, the DTM produced by the Ordinary Kriging with 5 meter elevation block average is used for the DSM construction.

Interpolation method	Accuracy assessment	Number of errors with more than 1.0 m in the area of interest	Visual assessment
Ordinary Kriging (5 m elevation block average)	1.00 m (at 80 percentile)	10	Able to represent the real landscape of the study area
Ordinary Kriging (100 m contour lines to point conversion interval)	1.17 m (at 80 percentile)	19	Able to represent the real landscape of the study area
TIN	N/A	7	Contains large number of artificial pits
ANUDEM	0.98 m (at 80 percentile)	21	Contains large number of artificial pits
Polynomial interpolation methods (2 nd Polynomial to 6 th Polynomial)	1.66 m to 1.74 m (at 80 percentile)	> 39	Smooth interpolated surface and with very general landscape of the study area

Table 3.3: Summary on DTMs quality assessment

3.1.4 Integrating natural terrain and man-made terrain

The DSM of the study area is constructed through combining the simulated man-made terrain and DTM. The DSMs are used to represent current and future situations of Naga City (see figure 3.3). The man-made features are simulated by integrating spatial data (e.g. roads, drainages, landuse or landcover), building footprints and detailed development plans. Feature 1 (Micro Drainage Project), 2 (elevated areas for commercial purposes) and 3 (new mall) are new features added onto the DSM of the current situation of Naga City. These features depicted future landuse plans which in general involve reclamation of the back-swamp areas.

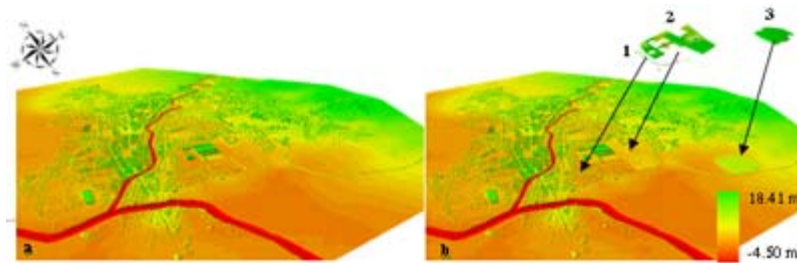


Figure 3.3: DSM represents current situation (a) and future situation of Naga City (b)

3.2 Flood modeling and flood model calibration

The 1D2D SOBEK flood model is used to simulate flood behavior which dominantly occurred due to substantial discharge of the Naga and thr Bicol River. This model requires incorporation of detailed floodplain terrain model and river cross sections, which both parts are represented by 2D and 1D model. The flood modeling covers 4 flood recurrence intervals namely 2, 5, 10 and 17.5 years return period flood. Flood depth and extent information of flood event triggered by the Supertyphoon Nanmadol is used in flood model calibration. Previous study had confirmed that the magnitude of this flood event is comparable to 10 years return period flood. The flood model calibration focuses on calibrating 2 parameters, 2 sets of building structures and 3 sets of surface roughness. In the flood model calibration process, it was found that, water velocity of rough surface building structure is higher compared to the solid block. In this case, the momentum of the overflowed water decreases when water-flow is obstructed by solid building block, which in turn decreases the ability of water to go farther. On the other hand, the rough surface building structures allows water to flow much farther and increase the flood extent. The obstruction has increased the flood depth and subsequently increase flood velocity at the edge of the buildings. Besides, there is no significant difference in flood depth for flood models with different set of manning coefficients. This suggests that, the effect of surface roughness on flood depth is not significant. Buildings in Naga City are quite dense for both residential and commercial areas, thus solid-block building structures is selected for further flood modelling.

3.3 Development impact assessment

Development impact assessment has combined 3 approaches in delivering messages on the changes of flood impact on current and future situations of Naga City. The first method concerns detailed changes in flood characteristics. This is followed by investigation on changes in human perception and flood hazard areas.

3.3.1 Changes in flood hazard areas

Flood hazard can be expressed by various combination of flood characteristics, for instance, flood depth and flood velocity based on research by Ramsbottom et al. (2003) (see figure 3.4). Hazard here specifically refers to the wading hazard of adults, children and also selected vehicles during flood. In this assessment, flood hazard maps for each simulation hour are created and integrated based on the worst case of hazard level at each pixel (see figure 3.5). The comparison on the flood hazard between current and after development situations is made by crossing both maps

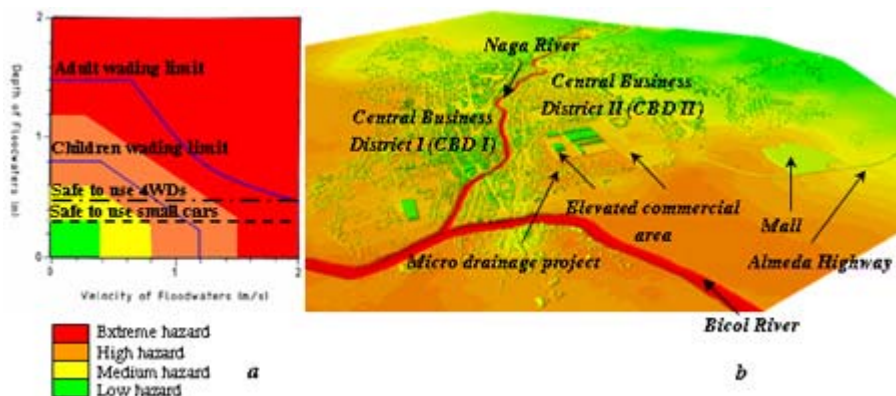


Figure 3.4: Graph that combines flood velocity and flood depth to define level of flood hazard (Ramsbottom et al., 2003) (a) and Major developments in Naga City (b)

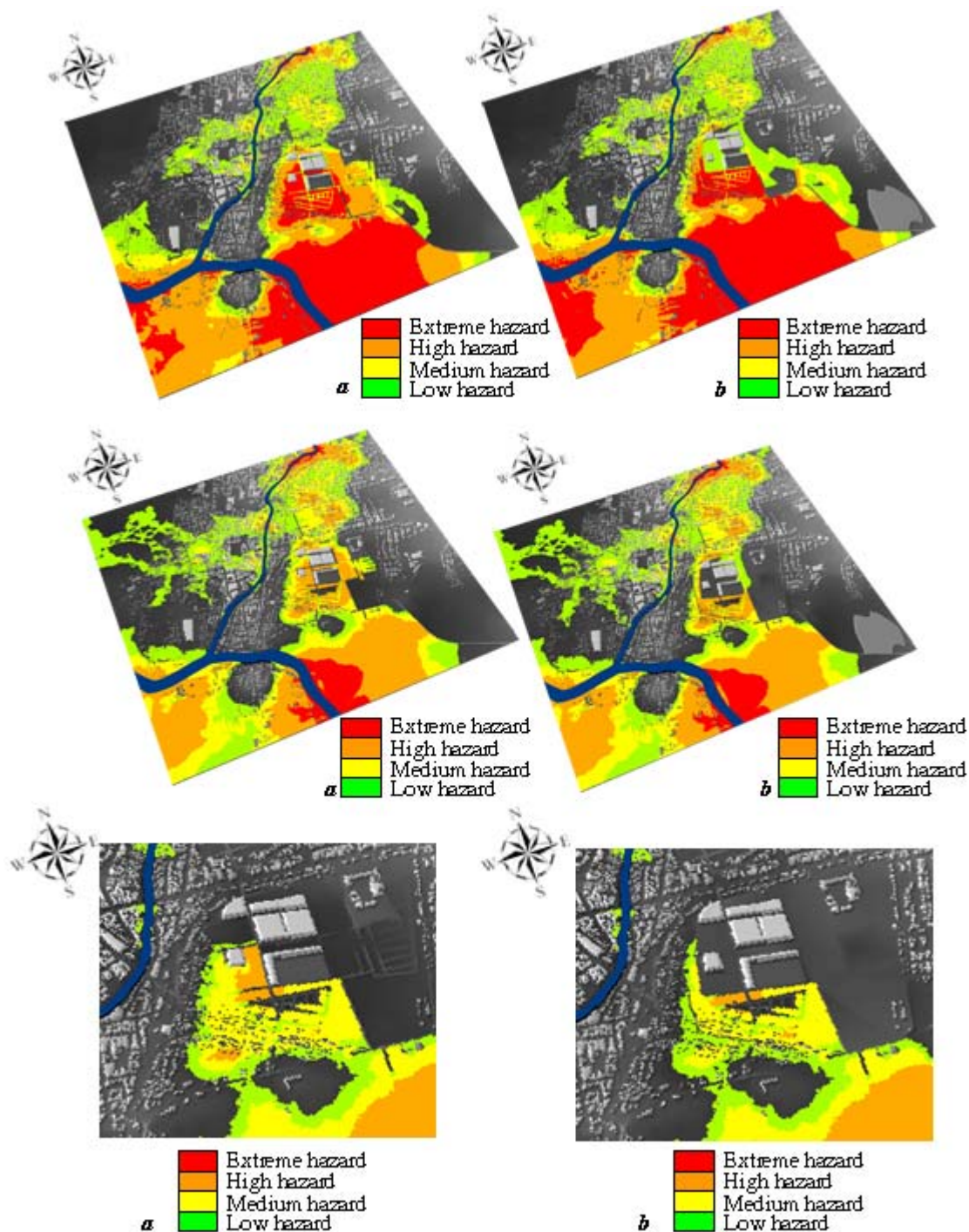


Figure 3.5: Flood hazard distribution for current (left) and future (right) situations for 17.5 years, 10 years and 5 years return period flood in Naga City

5.0 Results and discussions

5.1 DTM and DSM generation based on multi sources elevation data

Elevation data were derived from various sources, which characteristics differ in data scale, vertical accuracy and method of derivations. All datasets should be integrated and interpolated to produce the final DTM and DSM for flood modelling. However, each dataset covers different areas and contains

different type of landforms. In this case, complex terrain features for instance, rugged terrain and meandering river course will exhibit high variation in terrain elevation in a short distance. In this study, the step-by-step selection and integration of the elevation dataset had reduced the disagreement and overlapping dataset. Further than that, since the selection of the elevation dataset had based on the specific criteria, for instance the nominal accuracy, the best elevation value might be selected from all the datasets to represent the real terrain information. Nevertheless, disagreements between datasets could also exhibit high elevation variation in a short distance. Thus, reducing the “nugget effect” from the integrated dataset had consequently reduced the complexity of the terrain and also the disagreement between datasets. This is the trade-off that had been faced in this study; losing the detailed information while reducing the disagreement between the datasets. The next step was relied on the capability of commonly used terrain interpolation methods in generating the terrain model. Finally, the Ordinary Kriging had come out as the best terrain model, which had full-filled the DTM requirements for flood modelling purpose.

The other interpolation methods, for instance TIN and ANUDEM were quite sensitive with errors or noise in the integrated elevation dataset. Therefore, the final products of these interpolators contain artificial “pits” which in fact one of the big problems in hydrological applications. On the other hand, the DTMs produced by the polynomial interpolation methods lost the detailed landscape of the study areas. Furthermore, these DTMs contain large error compared to other interpolation methods and have large number of error point, with accuracy more than 1.0 m in the area of interest.

The 5 m resolution DTM is selected based on the nominal accuracy of the datasets and also the requirement in flood modelling for urban area. The 5 m DTM resolution is good enough to model the flood behaviour for individual structure and reclamation of small lands. Finally, the final DSM was generated by adding the man-made terrain onto the DTM.

5.2 Development impact assessment

Figure 3.6 shows detailed changes in flood hazard areas in Naga City for 17.5 and 10 years return period flood. According to figure 3.6 (b), for 10 years return period flood, it is about 92 percent of the areas with the positive impact are located in commercial area. Meanwhile, areas with negative impact are mostly in the residential and other commercial areas (figure 3.6 (a)). On the other hand, for 17.5 years return period flood (figure 3.6 (d)), commercial areas account about 96 percent of the total areas with positive impact. Besides, it is about 34 percent of the agriculture areas, 21 percent of the residential areas and 13.88 percent of the commercial areas are classified in negative impact areas.

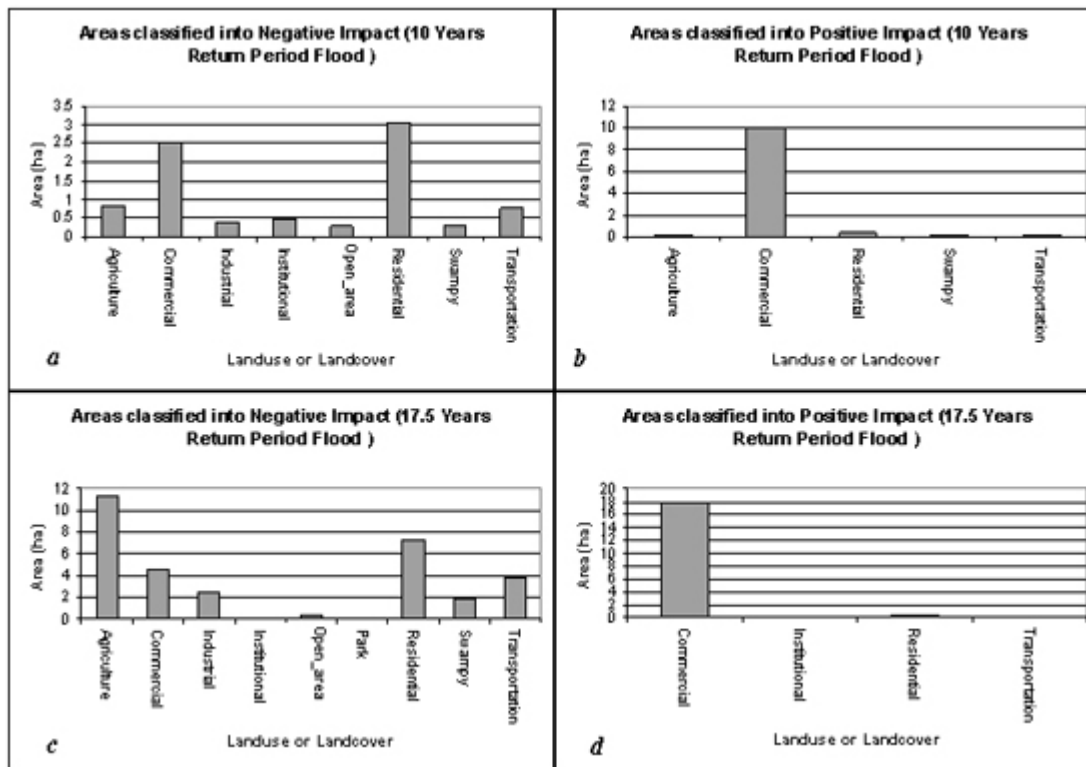


Figure 3.6: Areas classified into negative and positive impact (based on the hazard map) for 10 years return period flood (a) and (b) and 17.5 years return period flood (c) and (d)

6.0 Conclusions

Step by step multi-sources elevation data integration is one of the cheapest approaches in constructing terrain model without precise and expensive elevation data acquisition techniques for instance, LiDAR, Radar Interferometry and Aerial Photos. However, the integration of all the available elevation datasets should be made very carefully. In this case, the terrain updating process can be made through the conventional levelling and the compilation of recent and future development plans to be added onto the natural ground terrain model. This method certainly is very useful in developing countries which need rapid terrain information updating with less cost.

It must be realised that the development impact assessment doesn't intend to oppose any developments in Naga City. The assessment clearly aims at giving ideas on how flood behaviour would change after several major developments were carried out, especially in flood prone area. Thus the simulation results would give better ideas and understanding on flood behaviour which subsequently aids flood management and mitigation processes. Instead of that, this research has also given several simple methods in quickly detecting and assessing the detailed impact on flood behaviour.

Through this study, it was proved that, by elevating the ground terrain can only solve the flood problem in a particular area, but unfortunately this problem is transferred to another area. Small scale development, for instance the Micro Drainage Project in Barangay Triangulo had quite a pronounced role in changing the flood behaviour, especially in the small magnitude flood. The 17.5 and 10 years return period flood are quite large, thus the contribution of such small terrain changes can be neglected. In 2, 5, 10 and 17.5 years return period flood, the new Mall proposed to be constructed next to the Almeda Highway was not inundated by water. Furthermore, the Almeda Highway acted as a man-made levee or barrier and had efficiently obstructed the flood water, especially from the Bicol River.



Figure 3.7: Risk management cycle; the results of this study can be used in HAZARD mapping and MITIGATION process. Source: Smith (2001)

Development is necessary to provide current necessities and needs of the future generations. However, sustainable development may not be achieved without any effort in reducing the intensity of hazard as a result of developments. In this case, Naga City had taken many wise steps in managing hazards, for instance flood hazard mapping, wind hazard mapping and damage assessment, which are crucial for assessing potential loss and further actions to reduce it. In this study, it was proved that, the 1D2D flood modelling can be used to investigate the behaviour of floods and the flood impact after developments. Detailed investigation on the changes of flood characteristics could in fact reduce future loss and damage. On the other hand, it is also a wise initial step to reduce future investment on flood mitigation. Furthermore, several flood scenarios can be created and simulated in investigating the effectiveness of physical flood mitigation measures in reducing flood problems. Figure 3.7 shows that this study basically can be used in flood hazard mapping, flood risk assessment and mitigation phases.

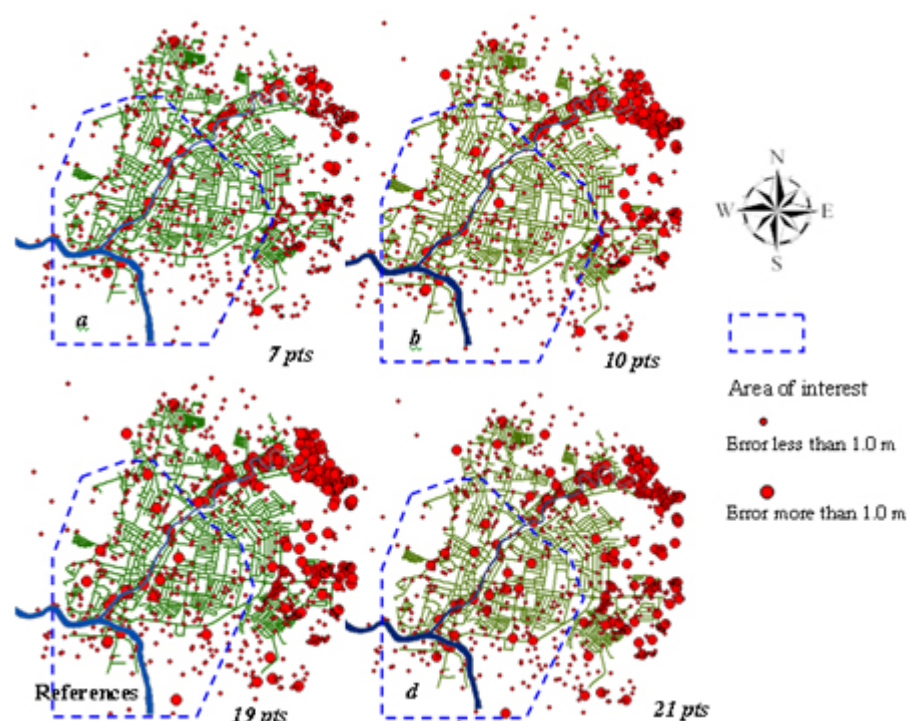
7.0 Appendices

	Source of data	Map scale and GPS positioning accuracy	Contour interval and the precision of geodetic levelling	Production year	Production method
1	Contour line south of Naga City	1:4000	0.25 m contour interval	1981	Aerial photography and ground control survey
2	Contour line for the whole Naga city	1: 1000	1.00 m contour interval	1981	N/A
3	Contour line from Naga Drainage Plan (1981)	1: 1000	0.25 m contour interval	1981	N/A
4	Contour line from CBD II development Plan	1: 1000	0.25 m contour interval	2000	Ground control survey and contour line interpolation
5	Spot height from topographical map south of Naga City	1:4000	N/A	1981	Ground control survey for map with scale of 1:4000
6	Spot height along roads from the Naga City Drainage Plan (1981)	1:1000	N/A	1981	Ground control survey
7	Spot height from the Drainage Plan in Triangulo (ground and drainage crown elevation)	1:1000	Less than 1 meter	2000	Ground control survey
8	Spot height from the Almeda highway plan (ground and final road elevation)	1:1000	Less than 1 meter	2000	Ground control survey
9	Spot height for the whole Bicol region	1: 10,000	N/A		Ground control survey
10	Field observation spot height	Less than 5m (GPS accuracy)	Less than 1 meter	2003	Ground control survey

Appendix 1: Source of elevation data for DTM generation

Interpolation techniques	Percentile	Error (m)	Mean	Std	Boundary (3 sigma)	Num of outlier
<i>Ordinary Kriging with 5-meter block average</i>	100	5.25	-0.03	1.02	3.06	> 1
	95	2.57	-0.04	0.72	2.16	> 1
	90	1.69	-0.04	0.72	2.16	None
	80	-1.00	0.00	0.39	1.17	None
	70	-0.66	0.02	0.28	0.84	None
	60	0.50	0.01	0.20	0.60	None
<i>Ordinary Kriging with 100 m line to point conversion distance interval</i>	100	4.98	-0.04	1.15	3.45	> 1
	95	-2.75	-0.02	0.87	2.61	> 1
	90	2.06	-0.04	0.68	2.04	> 1
	80	-1.17	0.00	0.45	1.35	None
	70	0.73	0.02	0.35	1.05	None
	60	0.56	0.03	0.27	0.81	None
<i>Triangulated Irregular Network (TIN)</i>	100	28.70	0.81	2.14	6.42	> 1
	95	4.61	0.41	0.80	2.40	> 1
	90	2.26	0.26	0.47	1.41	> 1
	80	0.88	0.12	0.19	0.57	> 1
	70	0.32	0.05	0.08	0.24	> 1
	60	0.15	0.02	0.04	0.12	> 1
<i>2nd Degree linear</i>	100	2.74	0.03	1.64	4.92	None
	95	-3.57	-0.06	1.29	3.87	None
	90	2.77	-0.05	1.09	3.27	None
	80	1.74	0.02	0.87	2.61	None
	70	-1.39	0.02	0.73	2.19	None
	60	1.12	0.01	0.59	1.77	None
<i>2nd Degree parabolic</i>	100	7.07	0.01	1.67	5.01	> 1
	95	-3.80	-0.10	1.29	3.87	None
	90	2.74	-0.11	1.11	3.33	None
	80	1.68	0.01	0.88	2.64	None
	70	1.36	0.02	0.74	2.22	None
	60	-1.11	-0.00	0.61	1.83	None
<i>2nd Degree polynomial</i>	100	6.74	0.03	1.64	4.92	> 1
	95	-3.57	-0.06	1.29	3.87	None
	90	-2.77	-0.05	1.09	3.27	None
	80	1.75	0.02	0.88	2.64	None
	70	-1.39	0.02	0.73	2.19	None
	60	1.12	0.01	0.60	1.8	None
<i>3rd Degree</i>	100	6.44	0.04	1.56	4.68	> 1
	95	-3.40	-0.03	1.21	3.63	None
	90	-2.64	-0.00	1.03	3.09	None
	80	1.66	0.05	0.80	2.4	None
	70	-1.27	0.43	0.42	1.26	None
	60	0.97	0.01	0.54	1.62	None
<i>4th Degree polynomial</i>	100	6.60	0.04	1.55	4.66	> 1
	95	3.43	-0.03	1.20	3.60	None
	90	2.58	-0.01	1.01	3.04	None
	80	1.66	0.04	0.79	2.36	None

Appendix 2: DTM accuracy given by the Percentile Accuracy Assessment method introduced by (Maune, 2001)



Appendix 3: Errors distribution for TIN (a), Ordinary Kriging with 5 m block average (b), Ordinary Kriging (100 m line to point conversion) (c) and ANUDEM (d); TIN has the least error within the study area, followed by block ordinary kriging with 5 m, Ordinary Kriging for 50 m line to point conversion interval and ANUDEM

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